

## A Plasticity-like Framework for Fracture Mechanics

A. Salvadori

CeSiA - Centro di studio e ricerca di sismologia applicata e dinamica strutturale,  
DICATA - Dipartimento di Ingegneria Civile, Architettura, Territorio e Ambiente  
Università di Brescia, via Branze 43, 25123 Brescia, Italy  
alberto.salvadori@ing.unibs.it

**Keywords:** Plasticity, fracture mechanics, quasi-static crack growth.

**Abstract.** An incremental description of (linear elastic) fracture mechanics is presented which shows a perfect analogy with plasticity theory. The formulation of a generic criterion stemming from the associated plasticity theory is presented and its feature discussed. The analogy between plasticity and quasi-static crack growth leads also to a new algorithm for crack propagation for an arbitrary number of cracks in multi-connected materials, which is driven by the increment of external actions. Stability of crack path under mode I loading is finally analyzed.

### Introduction

Fracturing process reveals three distinct phases [1]: loading without crack growth, stable crack growth an unstable crack growth. During crack advancing, energy dissipation takes place in the process-region, in the plastic region outside the process region, and eventually in the wake of plastic region. When the fracture process is idealized to infinitesimally small scale yielding, energy dissipation during crack growth is concentrated at the crack tip. This assumption together with linear elasticity is assumed in the present note, making use of Hooke's law without limitation of stress and strain magnitudes: the stress-strain fields in the crack tip vicinity is uniquely determined by the stress intensity factors (SIFs).

Similarly to the determination of the "elastic limit", the concept of incipient crack growth is difficult to identify: in both cases, the difficulty is solved by a convention. Onset of crack growth is governed theoretically by a local condition, describing when the process region reaches a critical state which, in most cases of engineering interest, is independent on body and loading geometry: this property is termed autonomy [2]. Several criteria, the Maximum Tensile Stress [3], the Maximum Shear Stress [4], the apparent Crack Extension Force [5], and the Strain Energy Density [6] to cite but a few, stem from the crack configuration "at the onset of propagation": they have been extensively represented in the SIFs plane  $K_1 - K_2$ . Many other criteria are grounded on the stress and strain fields in the "propagated configuration" as the crack elongation approaches zero from above, among them the Local Symmetry [7] and the Maximum Energy Release Rate [8, 9] criterion. It is natural therefore to analyze these criteria into a different plane, that in the rest of the paper will be named the Amestoy-Leblond plane.

Even if the total amount of stable crack growth does not obey the property of autonomy, being dependent on the plastic region about the crack tip, stable crack growth is ruled by local conditions at the process region. The onset of unstable crack growth is, on the contrary, a result of a global instability. Analogously to plasticity [10], the global quasi-static fracture propagation problem consists in seeking an expression of the crack propagation rate for all three phases of the fracturing process. The question can be posed in the following way: given the state of stress and the history of crack propagation (if any), express the crack propagation rate (if any) as a function of the stress and of the history. Indeed this path of reasoning seems quite natural: though, most of algorithm for crack propagation are designed in the opposite way: they express the external load history as a function of the crack propagation rate [11, 12]. Whereas this approach is quite easy, it is not optimal in evaluating

the critical point of the equilibrium path and further it seems to be unsuitable in the presence of many propagating cracks in multi-connected bodies.

For linear elastic fracture mechanics, the crack propagation problem is studied in section exploiting its analogy with plasticity theory. A maximum principle is stated, that expresses the maximum dissipation at the crack tip during propagation; from it, *associated flow rule and a propagation criteria for angle determination descend*. As an important implication, crack propagation angles from any aforementioned criteria can be eventually recovered *keeping the convexity of the safe equilibrium domain, the constraint  $K_1^* \leq K_1^C$ , the correct energy dissipation at the crack tip*. Consistency conditions lead to the formulation of an algorithm for crack advancing in section , which is *driven by the increment of external actions* (under the simplifying assumption of proportional loading) and *allows the evaluation of crack length increment and curvature at the crack tips of several cracks contemporarily advancing*. Stability of crack path under mode I loading, as it has been analyzed in [13], is recovered for slightly curved or kinked cracks and *extended to any crack propagation angle*.

Small strains and displacements hypothesis is assumed on a domain  $\Omega = \bigcup_{n=1}^N \Omega_n \subset \mathbb{R}^2$ , together with isotropic linear elastic constitutive law in all the  $N$  homogeneous closed domains  $\bar{\Omega}_n$ . Interfaces between domains are assumed to be rigid, i.e. relative displacements along each interface are not allowed. Loci  $\Upsilon_i, i = 1, 2, \dots,$  of possible displacement discontinuities  $w_i(\mathbf{x})$  are defined as usual - see [14] for details - inside of each domain  $\Omega$ : the issues of interface cracks and of intersection between moving cracks and interfaces fall beyond the scopes of the present note.

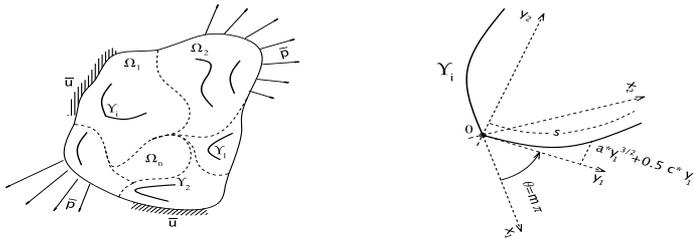


Figure 1: Notation.  $C$  denotes the curvature of the main branch at the crack tip, whereas  $a^*$  and  $C^*$  define the curvature of the elongated branch.

The structural response to the following quasi-static external actions is sought: tractions  $\bar{p}(\mathbf{x})$  on  $\Gamma_p \subset \partial\Omega$ , displacements  $\bar{u}(\mathbf{x})$  on  $\Gamma_u \subset \partial\Omega$ . They are all assumed to be *proportional*, i.e. that they vary only through multiplication by a time-dependent scalar  $k(t)$ , termed load factor, taken to be zero at initial time  $t_0 = 0$  when the cracks attained their initial length. In the present note, “time”  $t$  represents any variable which monotonically increases in the physical time and merely orders events; the mechanical phenomena to study are time-independent. Domain forces are assumed to be zero.

The notation of [15], see also figure 1, will be used. In their celebrated paper [15], Amestoy and Leblond established the general form of the expansion of the stress intensity factors (SIFs) in powers of the crack extension length  $s$ , for a crack propagating in a two-dimensional body along an arbitrary kinked (by an angle  $\theta = m\pi$ ) and curved path and evaluated the detailed form the functions of the geometric and mechanical parameters which appear in the expansion. Denoting with  $\mathbf{K} = \{K_1, K_2\}$

the SIFs vector, the expansion of  $\mathbf{K}$  at the extended crack tip in powers of  $s$  is of the general form:

$$\mathbf{K}(s) = \mathbf{K}^* + \mathbf{K}^{(1/2)} \sqrt{s} + \mathbf{K}^{(1)} s + O(s^{3/2}) \quad (1)$$

where  $\mathbf{K}^*$ ,  $\mathbf{K}^{(1/2)}$ ,  $\mathbf{K}^{(1)}$  are given componentwise (using the Einstein summation convention) by

$$K_p^* = F_{pq}(m) K_q \quad (2)$$

$$K_p^{(1/2)} = G_p(m) T + a^* H_{pq}(m) K_q \quad (3)$$

$$K_p^{(1)} = Z_p + I_{pq}(m) b_q + C J_{pq}(m) K_q + a^* Q_p(m) T + a^{*2} L_{pq}(m) K_q + C^* M_{pq}(m) K_q \quad (4)$$

In these equations,  $T$ , and the  $b_q s$  are the non singular stress and coefficients of the  $\sqrt{r}$  terms in the stress expansion at the original crack tip  $\mathbf{0}$ . The  $F_{pq}s$ ,  $G_p s$ ,  $H_{pq}s$ ,  $I_{pq}s$ ,  $J_{pq}s$ ,  $Q_p s$ ,  $L_{pq}s$ , and  $M_{pq}s$  are functions of the kink angle  $\theta$ , which are termed universal because they obey to the autonomy concept<sup>1</sup>; finally,  $Z_p$  depends on the geometry of  $\Omega$ .

### A plasticity framework for LEFM

The definition of a “safe equilibrium domain” and of the “onset of crack propagation” as its closure remain to the plasticity theory [10, 16]: they appear as the counterpart of the elastic domain and of the yield surface. Provided that merely the crack tip is considered as a material point, one is tempted to state that a crack tip is not going to propagate if the SIFs vector  $\mathbf{K}^*$  belongs to the set:

$$\mathbb{E} = \{ \{K_1^*, K_2^*\} \in \mathbb{R}_0^+ \times \mathbb{R} \mid \varphi(K_1^*, K_2^*) < 0 \} \quad (5)$$

which is termed the “safe equilibrium domain”. When  $\mathbf{K}^* \in \mathbb{E}$  the material<sup>2</sup> surrounding the crack tip is experiencing a purely linear elastic behavior, eventually corresponding to an elastic unloading. The boundary of  $\mathbb{E}$ ,  $\partial\mathbb{E}$ , is named the “onset of crack propagation surface”:

$$\partial\mathbb{E} = \{ \{K_1^*, K_2^*\} \in \mathbb{R}_0^+ \times \mathbb{R} \mid \varphi(K_1^*, K_2^*) = 0 \} \quad (6)$$

and vectors  $\mathbf{K}^* \notin \bar{\mathbb{E}}$  are ruled out. The definitions above implicitly label the SIFs vector as an internal force for the LEFM problem, conjugated to a not yet specified internal variable.

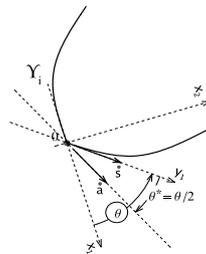


Figure 2: Definition of vector  $\hat{\mathbf{a}}$ . It is defined in the coordinate system  $\{y_1, y_2\}$  assuming that angle  $\theta^*$  is positive when counterclockwise, as usual. As a consequence, it always assumes the same absolute value and the opposite sign of  $\theta$ .

<sup>1</sup>see [2] but also the excellent description in [1]

<sup>2</sup>in the linear elastic modelization, whose effectiveness is restricted to the small-scale yielding approach

At all material points experiencing plastic deformations, mechanical dissipation  $\mathcal{D} > 0$  is induced; local dissipation inequality defines in plasticity (and more generally for standard dissipative systems) generalized strain rate as the conjugate to the generalized stress, as their product gives the rate of dissipation [10, 17]. In LEFM, mechanical dissipation is due to crack extension, for its irreversible nature [18]: it seems natural assuming as internal variable a quantity related to the quasi static crack tip velocity vector  $\dot{s}$ , defined as the vector oriented with axis  $y_1$  in figure 1 and with modulus equal to the quasi static velocity  $\left. \frac{ds}{dt} \right|_{s \rightarrow 0^+}$  as the crack elongation  $s$  approaches zero from above. The internal variable is here termed “dissipation rate” vector  $\dot{a}$  and is defined as in figure 2: it is related to  $\dot{s}$  by the orientation defined through the kinking angle  $\theta^* = \frac{1}{2} \theta$  and by its length, that will be proved to be equal to  $\left. \frac{ds}{dt} \right|_{s \rightarrow 0^+}$  too.

A maximum principle - termed  $\mathcal{D}$ -principle - for LEFM is postulated as follows:

For given dissipation rate vector  $\dot{a}$  among all possible SIFs on  $\mathbb{E}$ , the function

$$\mathcal{D}(\mathbf{k}^*; \dot{a}) = \mathbf{k}^* \cdot \begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix} \dot{a} \quad (7)$$

attains its maximum for the actual SIF vector  $\mathbf{K}^*$ :

$$\mathcal{D}(\mathbf{K}^*; \dot{a}) = \max_{\mathbf{k}^* \in \mathbb{E}} \mathcal{D}(\mathbf{k}^*; \dot{a}) \quad (8)$$

$\mathcal{D}$ -principle - analogously to maximum dissipation in plasticity [16] - implies: i) associative flow rule in the Amestoy-Leblond plane (normality law):

$$\dot{a} = \begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix} \frac{\partial \varphi}{\partial \mathbf{K}^*} \dot{\lambda} \quad (9)$$

ii) loading/unloading conditions in Kuhn-Tucker complementarity form:

$$\dot{\lambda} \geq 0, \quad \varphi \leq 0, \quad \dot{\lambda} \varphi = 0 \quad (10)$$

iii) convexity of safe equilibrium domain  $\mathbb{E}$ . The last of conditions (10) expresses the fact that  $\dot{\lambda}$  and  $\varphi$  are not simultaneously nonzero: crack extension (i.e.  $\dot{\lambda} > 0$ ) is possible when  $\varphi = 0$ , while negative  $\varphi$  implies that  $\dot{\lambda}$  must be zero, in which case the behavior is linear elastic. *Consistency condition* can be deduced from (10) as usual (see e.g. [10]); they read:

$$\text{When } \varphi = 0, \quad \dot{\lambda} \geq 0, \quad \dot{\varphi} \leq 0, \quad \dot{\lambda} \dot{\varphi} = 0 \quad (11)$$

Vectors  $\dot{a}$  and  $\dot{s}$  materialize the kinking angle  $\theta^* = \frac{1}{2} \theta$ , that can be obtained from the normality law as:

$$-\frac{\partial \varphi}{\partial K_1^*} \tan \theta^* = \frac{\partial \varphi}{\partial K_2^*} \quad (12)$$

The minus sign in the normality law (as well as the matrix  $\begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix}$  in the  $\mathcal{D}$  function) are required because when  $K_2 > 0$  the kinking angle  $\theta < 0$ , as already noticed in the previous section.

Consider as crack propagation criterion the Maximum Energy Release Rate in the form<sup>3</sup>:

$$\varphi(\mathbf{K}^*) = \frac{1}{2} \frac{1 - \nu^2}{E} \left( \|\mathbf{K}^*\|^2 - K_1^{C2} \right) \quad (13)$$

<sup>3</sup>The quadratic form (13) is not degree-one homogeneous. It can be shown that outcome (15) can be obtained by using a degree-one homogeneous form as well.

It holds:

$$\dot{\mathbf{a}} = \begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix} \frac{\partial \varphi}{\partial \mathbf{K}^*} \dot{\lambda} = \frac{1 - \nu^2}{E} \begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix} \mathbf{K}^* \dot{\lambda} \quad (14)$$

and

$$\mathcal{D}(\mathbf{K}^*; \dot{\mathbf{a}}) = \mathbf{K}^* \cdot \begin{bmatrix} 1 & 0 \\ 0 & -1 \end{bmatrix} \dot{\mathbf{a}} = \frac{1 - \nu^2}{E} \|\mathbf{K}^*\|^2 \dot{\lambda} \geq 0 \quad (15)$$

It can be therefore concluded that: *i.* function  $\mathcal{D}$  equals the energy dissipation at the crack tip due to an infinitesimal crack propagation  $\dot{\lambda} = \dot{s}$ ; consequently,  $\mathcal{D}$ -principle is the counterpart of the postulate of maximum plastic work; *ii.*  $\dot{\lambda} = \dot{s}$  is the actual “quasi-static crack propagation velocity” and  $\lambda = s$  will coincide with the total crack propagation, provided that  $\lambda = s = 0$  at the beginning of the crack propagation history.

### An algorithm for crack propagation

Consider a monotonic sequence of instants  $t_0 = 0, t_1, \dots, t_n, t_{n+1} = t_n + \Delta t$ . Assume all variables be known at  $t_n$ : the increments of the unknown are sought for the given variation over  $\Delta t$  of the external actions, governed by the variation  $\Delta k = k(t_{n+1}) - k(t_n)$  of the load factor. With the aim of readability, the index  $n$  will be omitted in this section when unnecessary, with the only exception of instant  $t_n$ .

Assume that  $J \geq 1$  crack tips are on the onset of propagation at  $t_n$ , that is for  $j = 1, 2, \dots, J$  the load factor  $k(t_n)$  is such that  $\{K_1^j, K_2^j\} \in \partial \mathbb{E}^j$ . Because all angles of propagation merely depends upon the ratio  $\alpha^j = \frac{K_2^j}{K_1^j}$ , they are assumed to be known at time  $t_n$  and termed  $\theta^j$ . Consistency conditions (10) can be invoked for the  $j$ -th crack elongation:

$$\dot{\varphi}^j = \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left( \frac{\partial K_i^*}{\partial k} \dot{k}(t_n) + \sum_{h=1}^J \frac{\partial K_i^*}{\partial a^h} \dot{a}^h(t_n) \right) \Bigg|_{\varphi^j=0} = 0 \quad j = 1, 2, \dots, J \quad (16)$$

The linear system of equations (16) relates the  $J$  crack elongation “velocities”  $\dot{a}^h$  to the variation of the load factor  $\dot{k}$  at time  $t_n$ , which drives the loading process and can be assumed to be given. In order to achieve a more effective algorithm, an arc-length procedure [19] can be set up instead of assuming a given load factor variation, with the arc-length eventually adapted by the curvature of the equilibrium path. In both cases, the constraint  $\dot{a}^h > 0$  for all  $h = 1, 2, \dots, J$  avoids troubles in the choice of the sign of variation  $\dot{k}$ .

In equation (16):  $K_i^*$  is always referred to the  $j$ -th crack, because any crack propagation criteria at a crack tip is merely dependent on SIFs at the same crack tip: for the sake of clearness the apex  $j$  is omitted for  $K_i^*$ ;  $\frac{\partial \varphi^j}{\partial K_i^*}$  depends on the selected criteria;  $\frac{\partial K_i^*}{\partial k}$  is trivial because for given crack lengths the global behavior is purely linear elastic: therefore

$$\frac{\partial K_i^*}{\partial k} \Bigg|_{\varphi^j=0} = \frac{K_i^*}{k} \Bigg|_{\varphi^j=0}$$

factors  $\frac{\partial K_i^*}{\partial a^h}$  depend on the global elastic problem and, besides intrinsic difficulties related to expansion (1), their evaluation is yet an on going research topic [20]. A way to circumvent such a drawback is assuming an expansion for  $a^h$  with respect to  $k$  at time  $t_n$  in the following form:

$$a^h(t) - a^h(t_n) = \sum_{\omega=1}^{\Omega} Q_{\omega}^h(a^1(t_n), \dots, a^J(t_n)) [k(t) - k(t_n)]^{\omega} \quad t > t_n \quad (17)$$

in which the maximum number of terms  $\Omega$  is suitably selected. Assuming for instance  $\Omega = 1$  leads to:

$$\begin{aligned} \varphi^j &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left( \frac{K_i^*(t_n)}{k(t_n)} + \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} \frac{\partial a^h}{\partial k} \Big|_{\varphi^j=0} + \frac{\partial K_i^*}{\partial a^j} \frac{\partial a^j}{\partial k} \Big|_{\varphi^j=0} \right) \\ &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left( \frac{K_i^*(t_n)}{k(t_n)} + \left[ \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_1^h + \frac{\partial K_i^*}{\partial a^j} Q_1^j \right]_{\varphi^j=0} \right) \\ &= 0 \quad j = 1, 2, \dots, J \end{aligned}$$

Taking into account of expansions (1) and (17), one gets:

$$\begin{aligned} \varphi^j &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left[ \frac{K_i^*(t_n)}{k(t_n)} + \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_1^h + \left( \frac{K_i^{(1/2)}}{2\sqrt{a^j(t) - a^j(t_n)} + K_i^{(1)}} \right) Q_1^j \right]_{\varphi^j=0} \\ &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left[ \frac{K_i^*(t_n)}{k(t_n)} + \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_1^h + K_i^{(1/2)} \frac{\sqrt{Q_1^j}}{2\sqrt{k(t) - k(t_n)}} + K_i^{(1)} Q_1^j \right]_{\varphi^j=0} \\ &= 0 \quad j = 1, 2, \dots, J \end{aligned}$$

The expression above states that coefficient  $Q_1^j$  of the linear contribution in expansion (17) at  $j$ -th crack tip is non vanishing if and only if<sup>4</sup>  $\mathbf{K}^{(1/2)^j} = 0$ , in which case:

$$\sum_{h \neq j} \frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \frac{\partial \mathbf{K}^*}{\partial a^h} Q_1^h + \frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^{(1)} Q_1^j = -\frac{1}{k(t_n)} \frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^* \quad j = 1, 2, \dots, J \quad (18)$$

According to equation (18),  $Q_1^j$  depends also on  $Q_1^h$ . If  $\frac{\partial \mathbf{K}^*}{\partial a^h}$  shows an  $o(1)$  dependency on  $k(t) - k(t_n)$  (at this moment this is still an open problem) it comes out:

$$Q_1^j = -\frac{1}{k(t_n)} \frac{\frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^*}{\frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^{(1)}} \quad j = 1, 2, \dots, J \quad (19)$$

Assuming  $\Omega = 2$  and taking  $\mathbf{K}^{(1/2)^j} \neq 0$  for all  $j = 1, 2, \dots, J$  leads to<sup>5</sup>:

$$\begin{aligned} \varphi^j &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left( \frac{K_i^*(t_n)}{k(t_n)} + \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} \frac{\partial a^h}{\partial k} \Big|_{\varphi^j=0} + \frac{\partial K_i^*}{\partial a^j} \frac{\partial a^j}{\partial k} \Big|_{\varphi^j=0} \right) \\ &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left( \frac{K_i^*(t_n)}{k(t_n)} + 2(k(t) - k(t_n)) \left[ \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_2^h + \frac{\partial K_i^*}{\partial a^j} Q_2^j \right]_{\varphi^j=0} \right) \\ &= 0 \quad j = 1, 2, \dots, J \end{aligned}$$

Assuming further that  $\frac{\partial K_i^*}{\partial a^h}$  shows no singular behavior when  $h \neq j$  and taking into account of expansion

<sup>4</sup>This result corresponds to assumption (100) at page 492 in [15].

<sup>5</sup>The general case  $\mathbf{K}^{(1/2)^j} \neq 0$  only for some  $j$  is a trivial extension of the following.

sions (1) and (17), one gets:

$$\begin{aligned} \varphi^j &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left[ \frac{K_i^*(t_n)}{k(t_n)} + 2 \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_2^h (k(t) - k(t_n)) + K_i^{(1/2)} Q_2^j \frac{k(t) - k(t_n)}{\sqrt{a^j(t) - a^j(t_n)}} \right]_{\varphi^j=0} \\ &= \dot{k}(t_n) \sum_{i=1}^2 \frac{\partial \varphi^j}{\partial K_i^*} \left[ \frac{K_i^*(t_n)}{k(t_n)} + 2 \sum_{h \neq j} \frac{\partial K_i^*}{\partial a^h} Q_2^h (k(t) - k(t_n)) + K_i^{(1/2)} \sqrt{Q_2^j} \right]_{\varphi^j=0} \\ &= 0 \quad j = 1, 2, \dots, J \end{aligned}$$

By neglecting the higher order term  $k(t) - k(t_n)$ , it comes out:

$$\sqrt{Q_2^j} = -\frac{1}{k(t_n)} \frac{\frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^*}{\frac{\partial \varphi^j}{\partial \mathbf{K}^*} \cdot \mathbf{K}^{(1/2)}} \quad j = 1, 2, \dots, J \quad (20)$$

According to equation (20),  $Q_2^j$  depends merely on the load factor  $k(t_n)$  and on quantities pertaining to the  $j$ -th crack tip: the presence of all other cracks is reflected by vectors  $\mathbf{K}^*(t_n)$  and  $\mathbf{K}^{(1/2)}$ . As this last term depends on the T-stress, so does  $Q_2^j$ ; if a straight elongation is considered, i.e.  $a^* = 0$  in equation (3),  $Q_2^j$  can be evaluated from (20).

In the more general case,  $a^*$  can be evaluated exploiting the normality law (12). In view of expansion (1), equation (12) becomes:

$$-\left(K_1^* + K_1^{(1/2)} \sqrt{s}\right) \tan \theta^* = K_2^* + K_2^{(1/2)} \sqrt{s}$$

whence the *zero*-order outcome:

$$\tan \theta^* = -\frac{K_2^*}{K_1^*}$$

The  $\frac{1}{2}$ -order equation:

$$-K_1^{(1/2)} \tan \theta^* = K_2^{(1/2)}$$

allows the evaluation of  $a^*$  in view of identity (3)

$$a^* = -\frac{G_2(\theta) - G_1(\theta) \tan \theta^*}{H_{1q}(\theta) K_q \tan \theta^* - H_{2q}(\theta) K_q} T \quad (21)$$

using the Einstein summation notation.

### Concluding remarks

Within the present note, a relation between the angle of propagation  $\theta$  and the angle  $\theta^*$  of the form  $\theta = 2\theta^*$  has been assumed. Indeed, a certain arbitrary is nested in this choice: it is the same degree of freedom actually present in the choice of the crack propagation criteria. A more involved relation  $\theta = \zeta(\theta^*)$  could be proposed in order to recover the crack propagation angle  $\theta$  predicted by any criteria: in this way, the degree of freedom in selecting a crack propagation criteria is transposed in the selection of mapping  $\zeta$ . As long as the remaining part of the plasticity analogy is kept, in particular the maximum energy release rate criterion for the safe equilibrium domain (13), the maximum principle (7) and its descending outcome (15), any selection of  $\zeta$  keeps the convexity of the safe equilibrium domain in the  $K_1 - K_2$  as well as in the Amestoy-Leblond d planes, the respect of the constraint  $K_1^* \leq K_1^C$  in mixed mode crack propagation, the correct energy dissipation at the crack tip.

The topic of the present note shows promising features and developments. Extending to linear elastic fracture mechanics the enormous amount of knowledge pursued in the last decades in plasticity is fascinating indeed, on several perspectives: theoretical, computational, educational.

## References

- [1] K.B. Broberg. *Cracks and fracture*. Academic Press, 1999.
- [2] G.I. Barenblatt. On equilibrium cracks forming during brittle fracture (in russian). *Prikladnaya Matematika i Mekhanika*, 23:434444, 1959. See also, *The mathematical theory of equilibrium cracks in brittle fracture*, Advances in appl. mech., 7 55-129 (1962).
- [3] G. Erdogan and G.C. Sih. On the crack extension in plates under plane loading and trasverse shear. *ASME J. Basic Engng*, 85:519527, 1963.
- [4] A. Otsuka, K. Mori, and T. Miyata. The condition of fatigue crack growth in mixed mode condition. *Eng. Fracture Mech.*, 7:429439, 1975.
- [5] H.C. Strifors. A generalized force measure of conditions at crack tips. *Int. J. Solids Structures*, 10:1389 1404, 1973.
- [6] G.C. Sih. Strain-energy-density factor applied to mixed-mode crack problems. *Int. J. Fracture*, 10:305321, 1973.
- [7] R.V. Goldstein and R.L. Salganik. Brittle fracture of solids with arbitrary cracks. *Int. J. Fract.*, 10:507523, 1974.
- [8] A.A. Griffith. The phenomena of rupture and flow in solids. *Phil. Trans. R. Soc.*, 221:163198, 1921.
- [9] C.W. Wu. Maximum-energy-release-rate criterion applied to a tension-compression specimen with crack. *Journal of Elasticity*, 8:235257, 1978.
- [10] W. Han and B.D. Reddy. *Plasticity*. Springer-Verlag, New York, 1999.
- [11] T.J. Stone and I. Babuska. A numerical method with a posteriori error estimation for determining the path taken by a propagating crack. *Comp. Methods Appl. Mech. Eng.*, 160:245271, 1998.
- [12] M.H. Aliabadi, C.A. Brebbia, and V.Z. Parton. *Static and dynamic fracture mechanics*. Computational mechanics publications, 1994.
- [13] B. Cotterell and J.R. Rice. Slightly curved or kinked cracks. *International Journal of Fracture*, 16:155169, 1980.
- [14] A. Salvadori and L.J. Gray. Analytical integrations and SIFs computation in 2D fracture mechanics. *Int. J. Numer. Methods Eng.*, 70:445495, 2007.
- [15] M. Amestoy and J.B. Leblond. Crack paths in plane situations -ii. detailed form of the expansion of the stress intensity factors. *International Journal of Solids and Structures*, 29:465501, 1992.
- [16] [16] J.C. Simo and T.J.R. Hughes. *Computational inelasticity*. Springer-Verlag, New York, 1998.
- [17] Q.S. Nguyen. *Stability and nonlinear solid mechanics*. John Wiley and Sons B.V., 2000.
- [18] J.R. Rice and D.C. Drucker. Energy changes in stressed bodies due to void and crack growth. *Int. J. of Fracture*, 3:1927, 1967.
- [19] E. Riks. An incremental approach to the solution of snapping and buckling problems. *Int. J. Solid Structures*, 15:529551, 1979.
- [20] C.G. Hwang and A.R. Ingraffea. Virtual crack extension method for calculating the second order derivatives of energy release rates for multiply cracked systems. *Engng. Fracture Mechanics*, 74(9):14681487, 2007.